

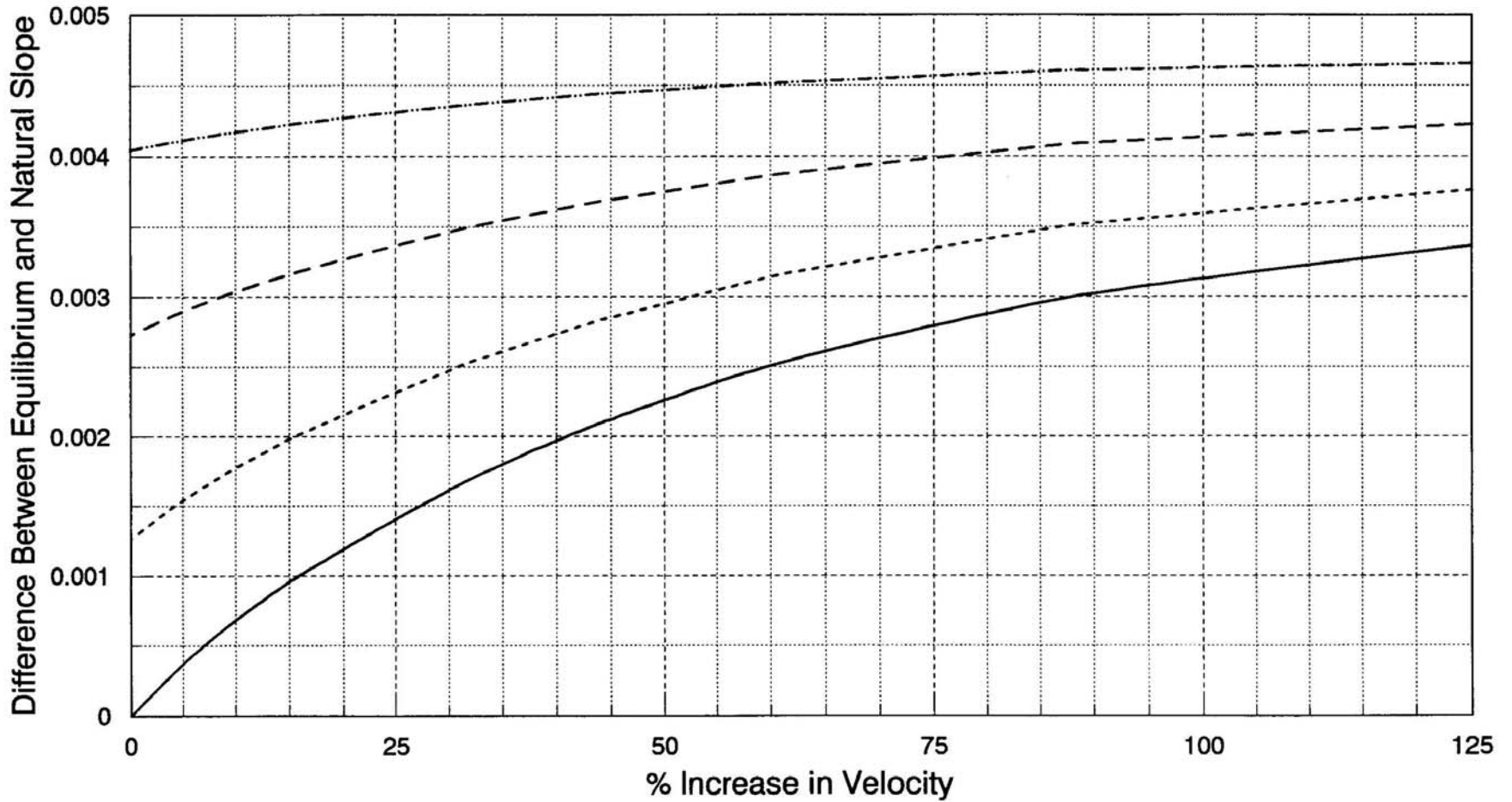
APPENDIX C

Sedimentation Design Curves

Equilibrium Slope	C-1A-C
Stabilization Requirements	C-2
General Degradation	C-3
Pier Local Scour	C-4
Angle of Attack Multiplying Factors	C-5
Abutment Local Scour	C-6
Bend Scour	C-7A-C
Longitudinal Extent of Bend Scour	C-8
Bedform Height	C-9
Riprap Rock Size	C-10
Limiting Deposit Velocity	C-11A-B
Sedimentation Design Curves Source Equations	C-12-14

EQUILIBRIUM SLOPE

Natural Slope = 0.005

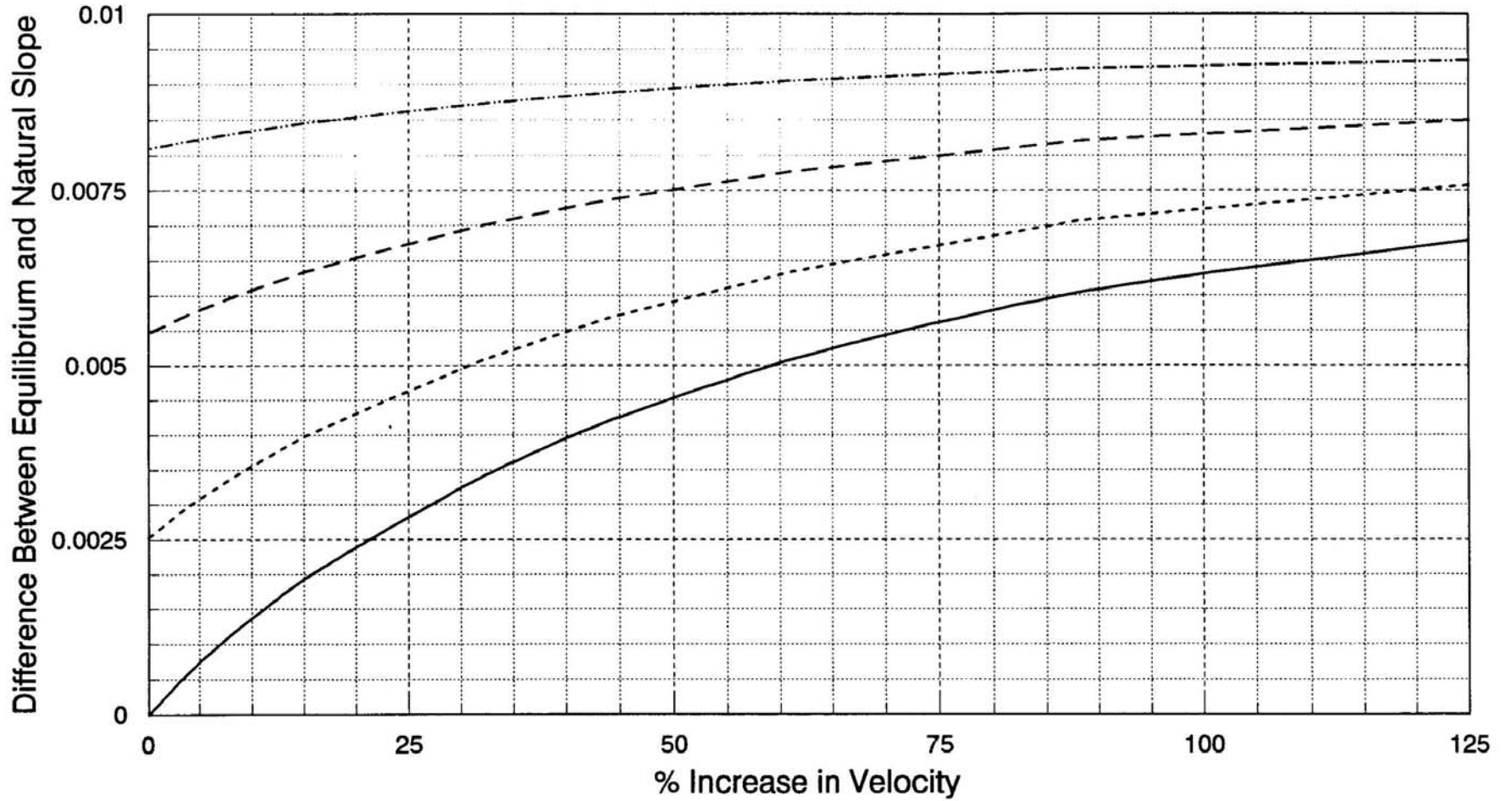


No Reduction in Sediment Supply 1/3 Reduction in Sediment Supply

2/3 Reduction in Sediment Supply 90% Reduction in Sediment Supply

EQUILIBRIUM SLOPE

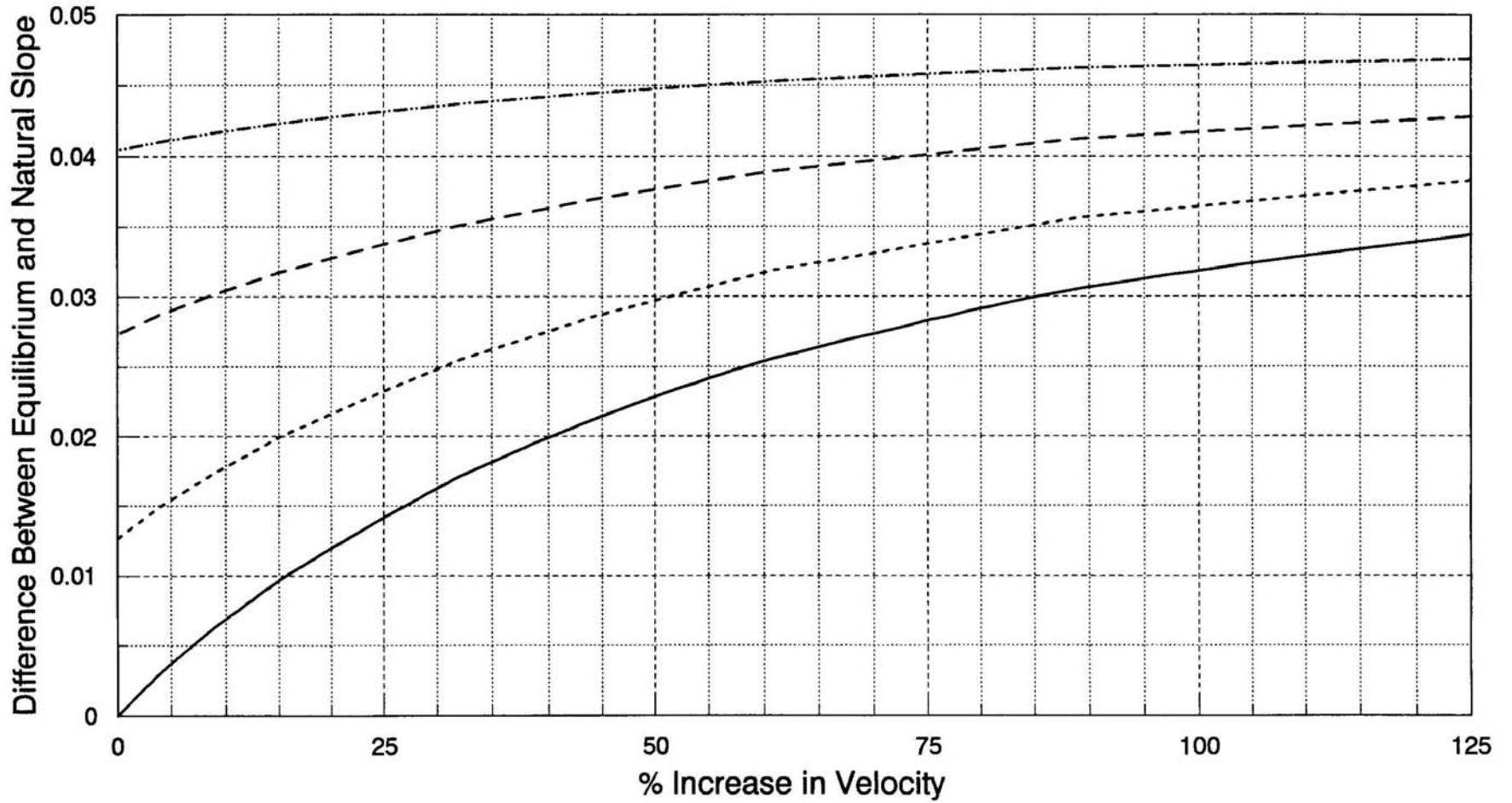
Natural Slope = 0.01



No Reduction in Sediment Supply	1/3 Reduction in Sediment Supply
_____	-----
2/3 Reduction in Sediment Supply	90% Reduction in Sediment Supply
-----

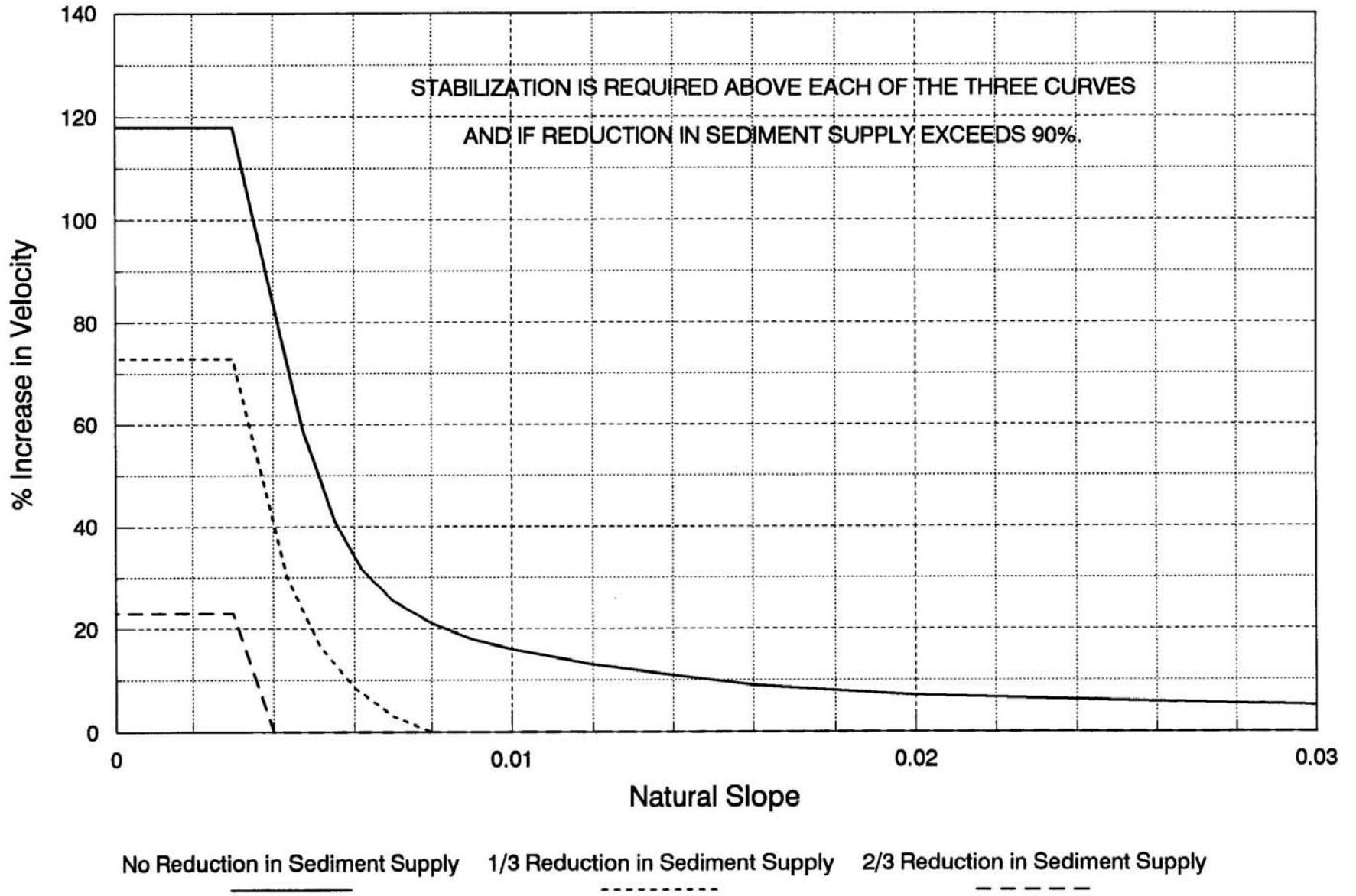
EQUILIBRIUM SLOPE

Natural Slope = 0.05

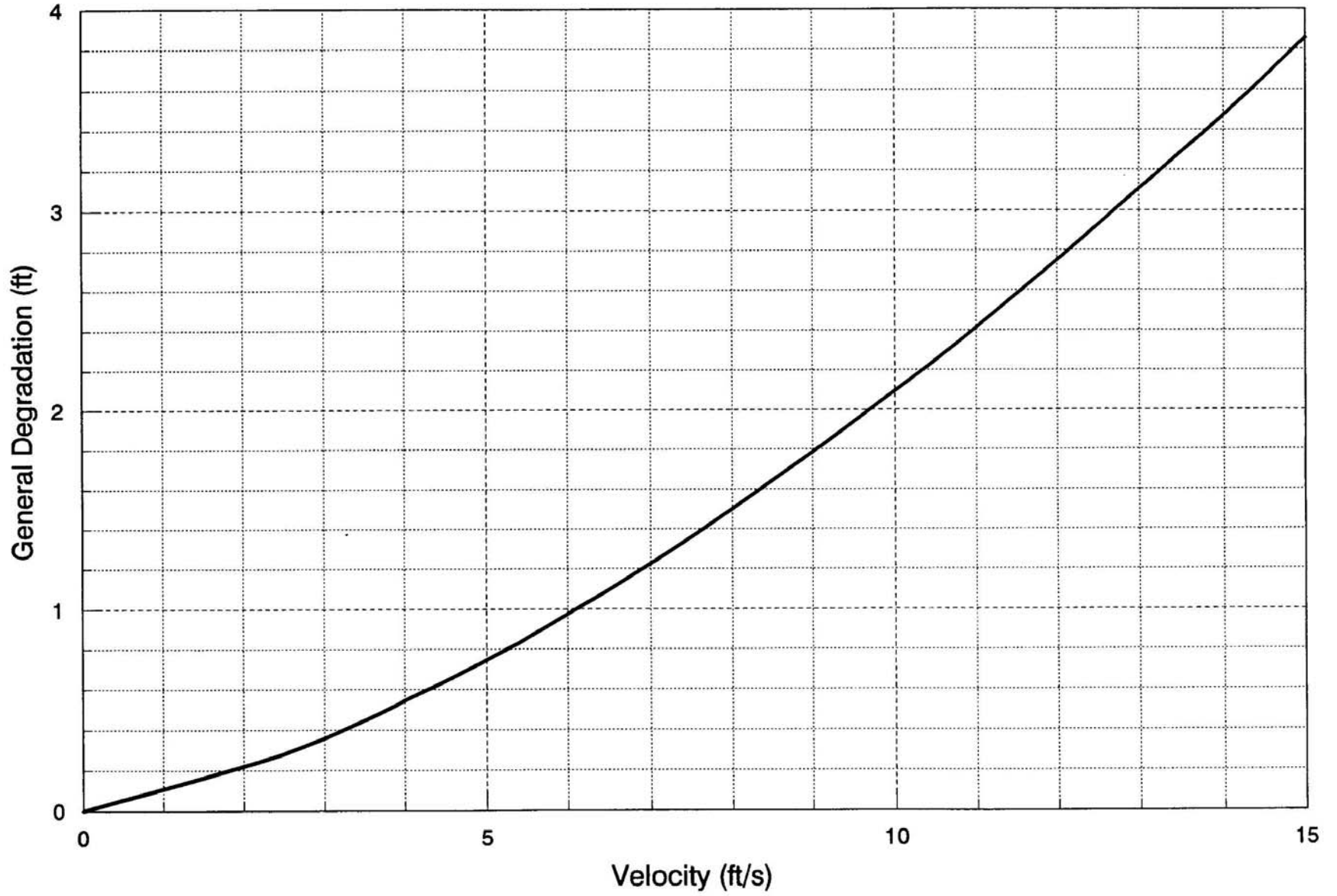


No Reduction in Sediment Supply	1/3 Reduction in Sediment Supply
_____
2/3 Reduction in Sediment Supply	90% Reduction in Sediment Supply
-----	-.-.-.-.

STABILIZATION REQUIREMENTS

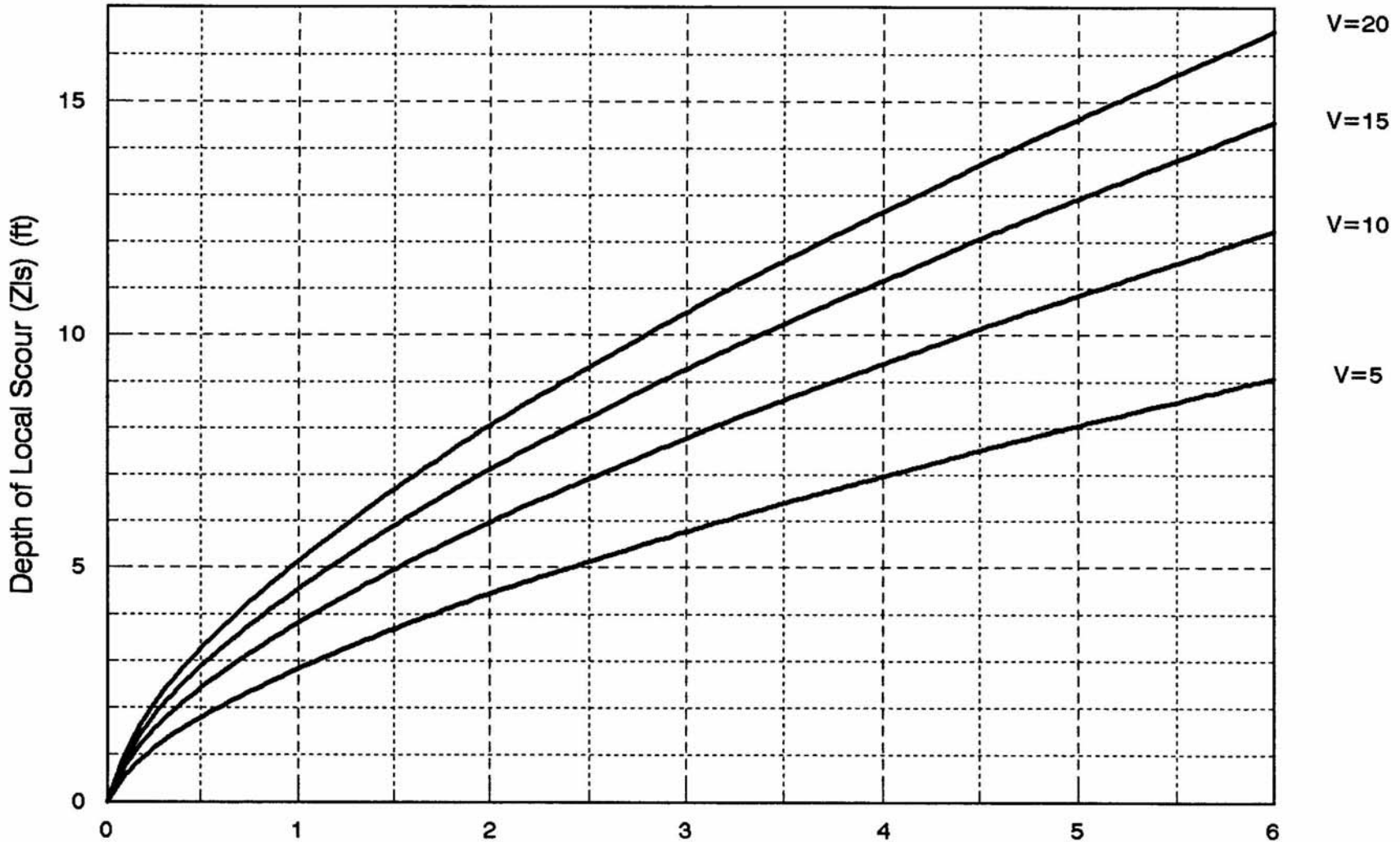


GENERAL DEGRADATION



PIER LOCAL SCOUR

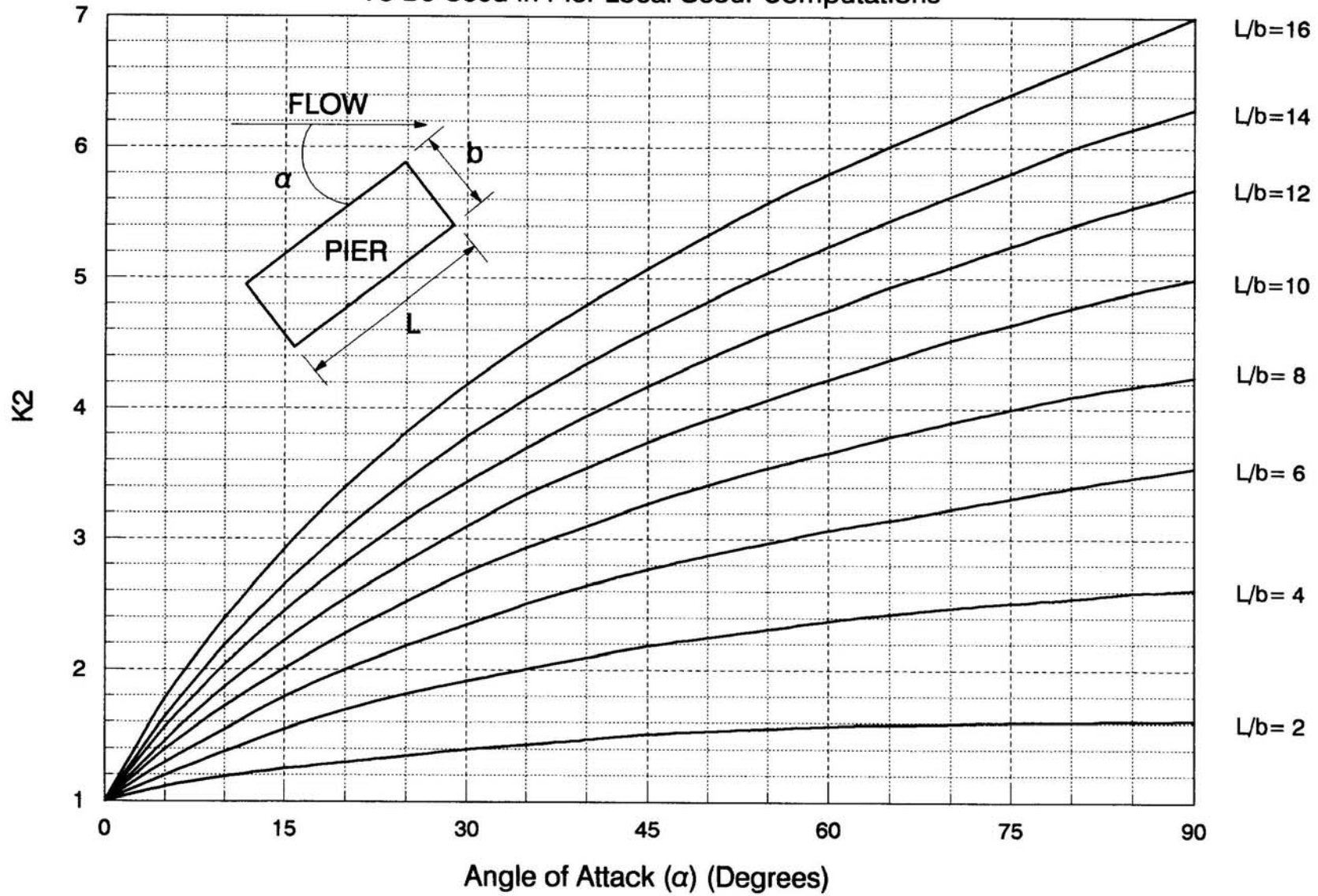
For Square Nose Piers



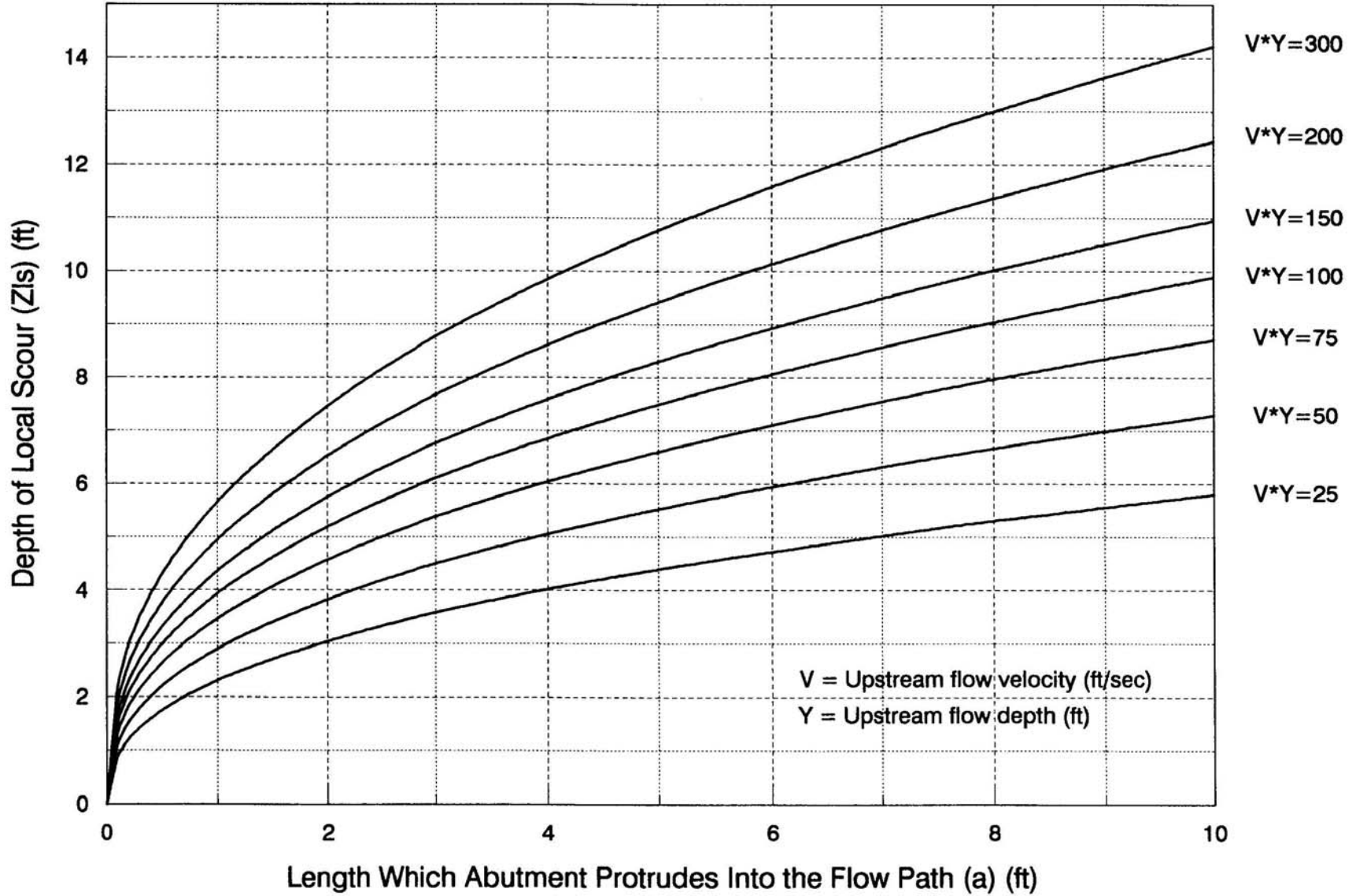
Use graph for depth less than 10 feet.
 Use equation on page Q-12 for deeper flows V = Upstream flow velocity (ft/sec)

MULTIPLYING FACTORS FOR ANGLE OF ATTACK

To Be Used In Pier Local Scour Computations

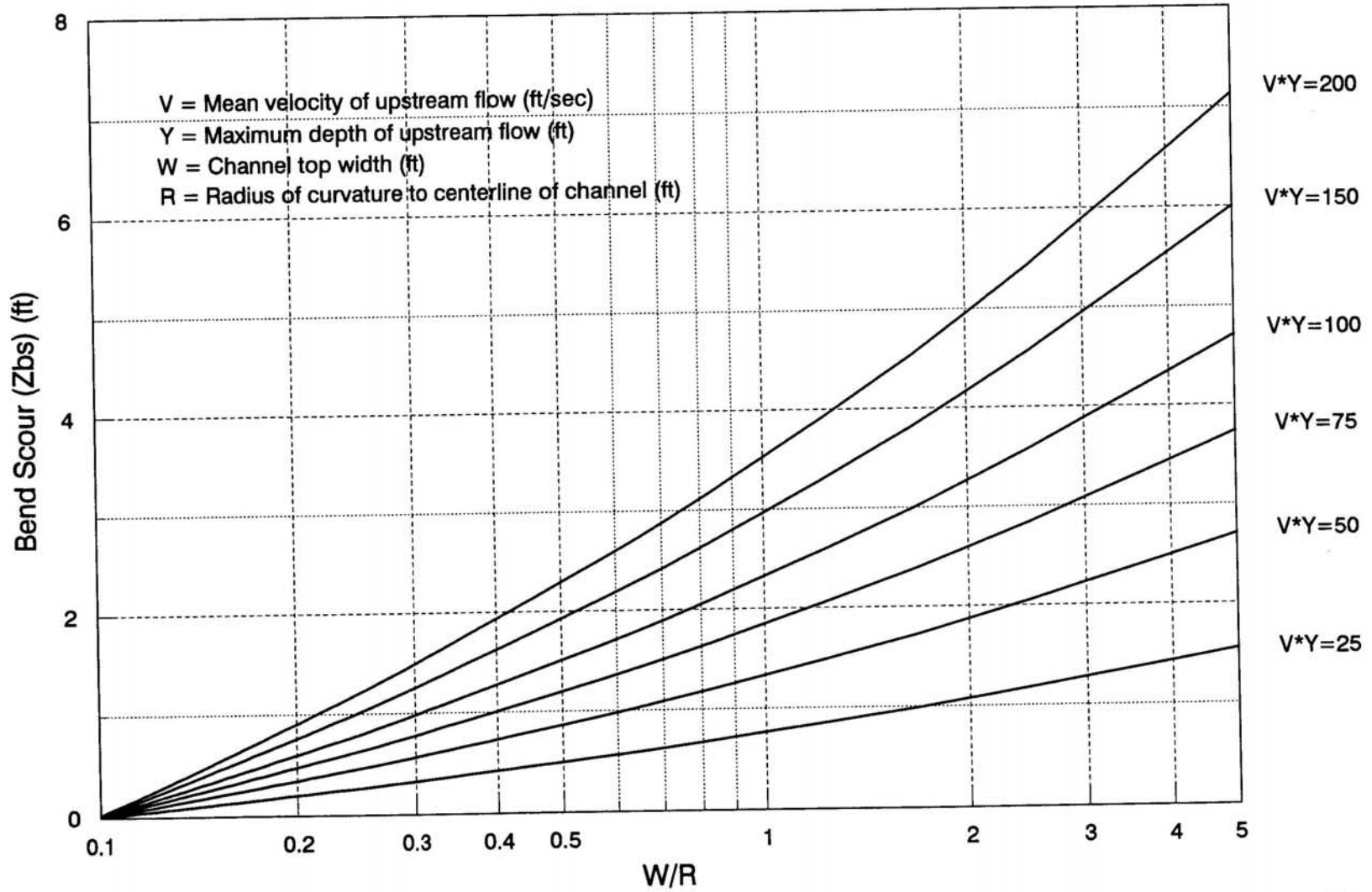


ABUTMENT LOCAL SCOUR



BEND SCOUR

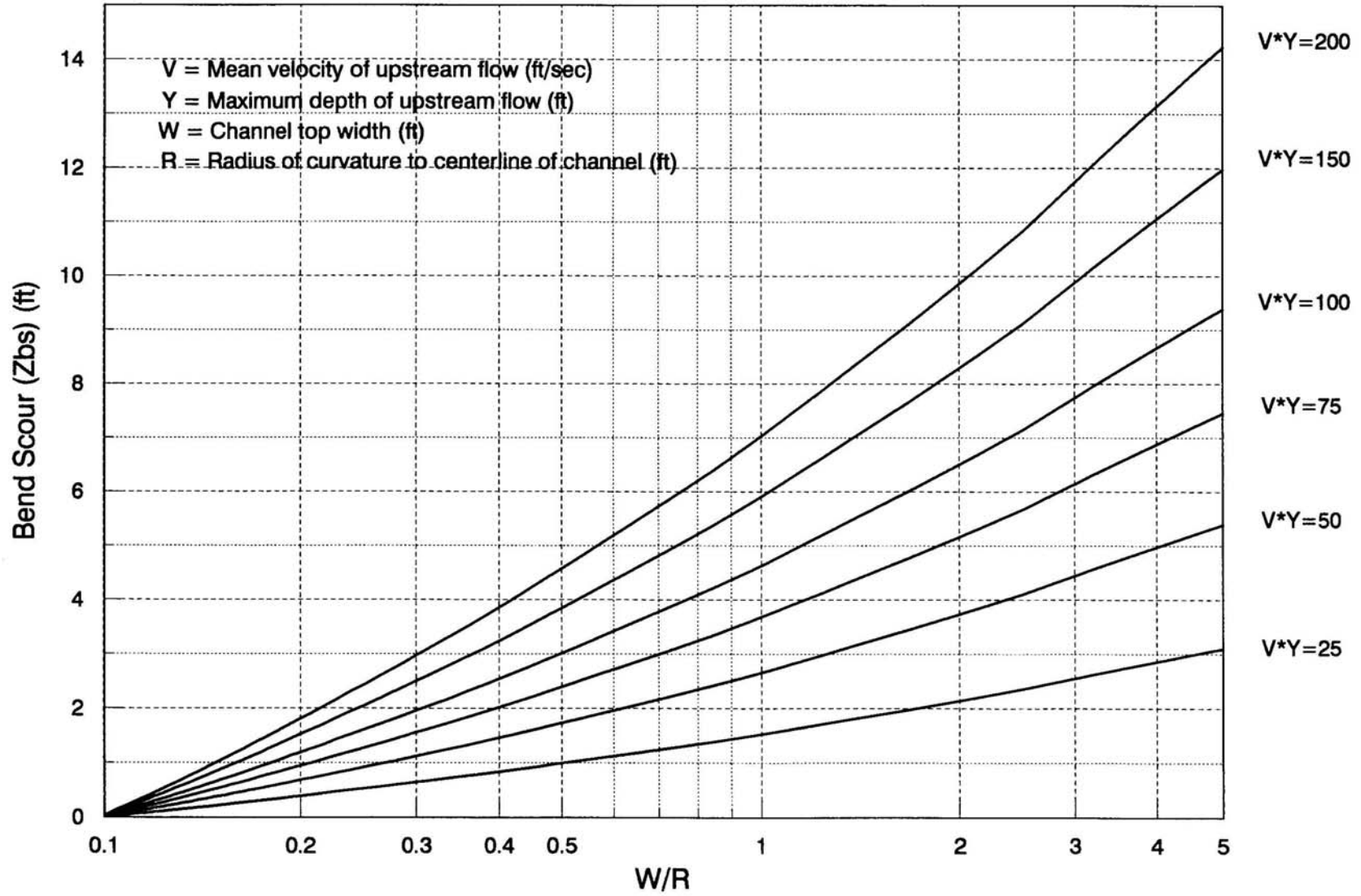
Energy Slope (S_e) = 0.1



For S_e other than 0.1, 0.01, and 0.001 use linear interpolation or extrapolation

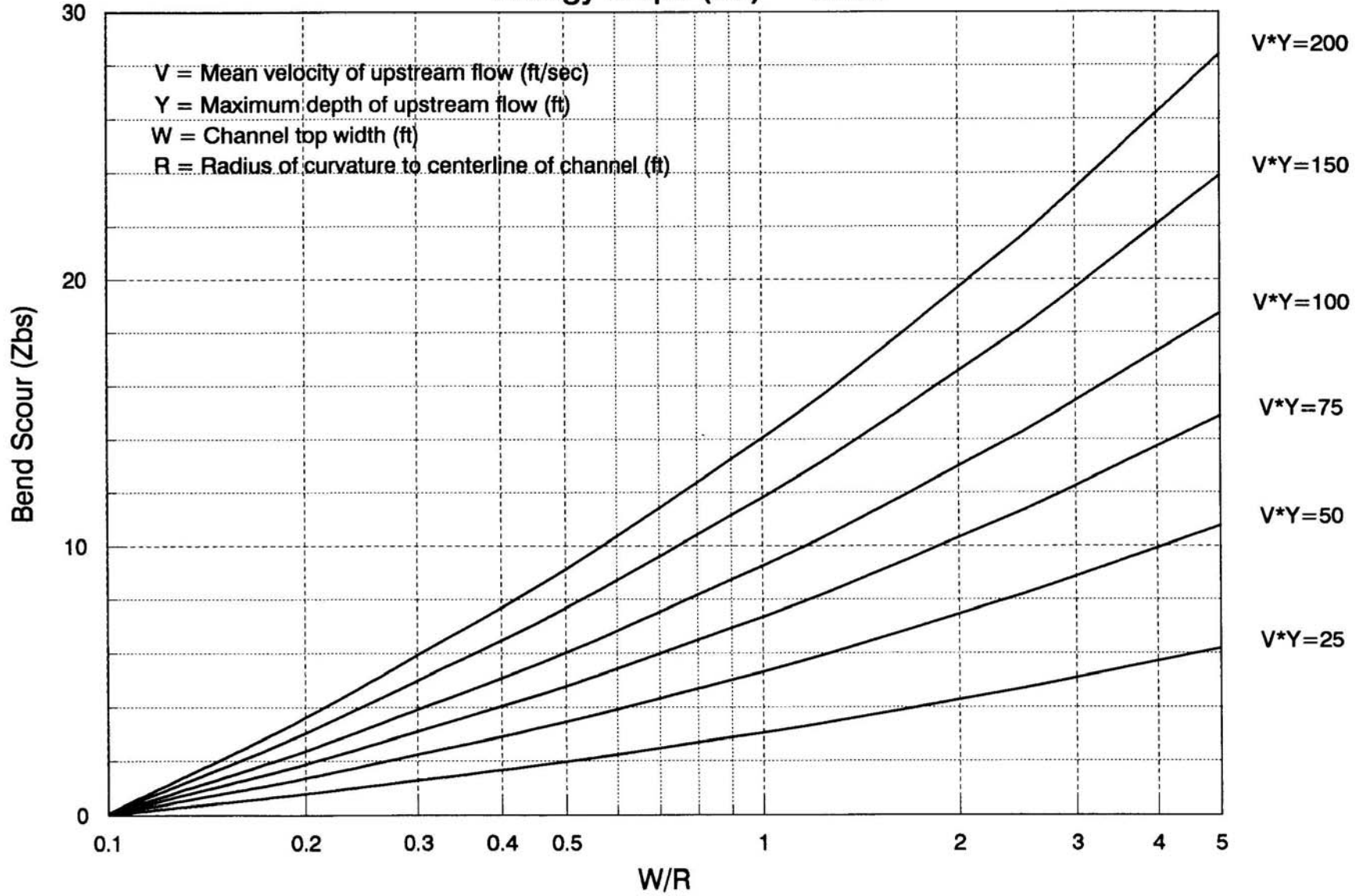
BEND SCOUR

Energy Slope (S_e) = 0.01

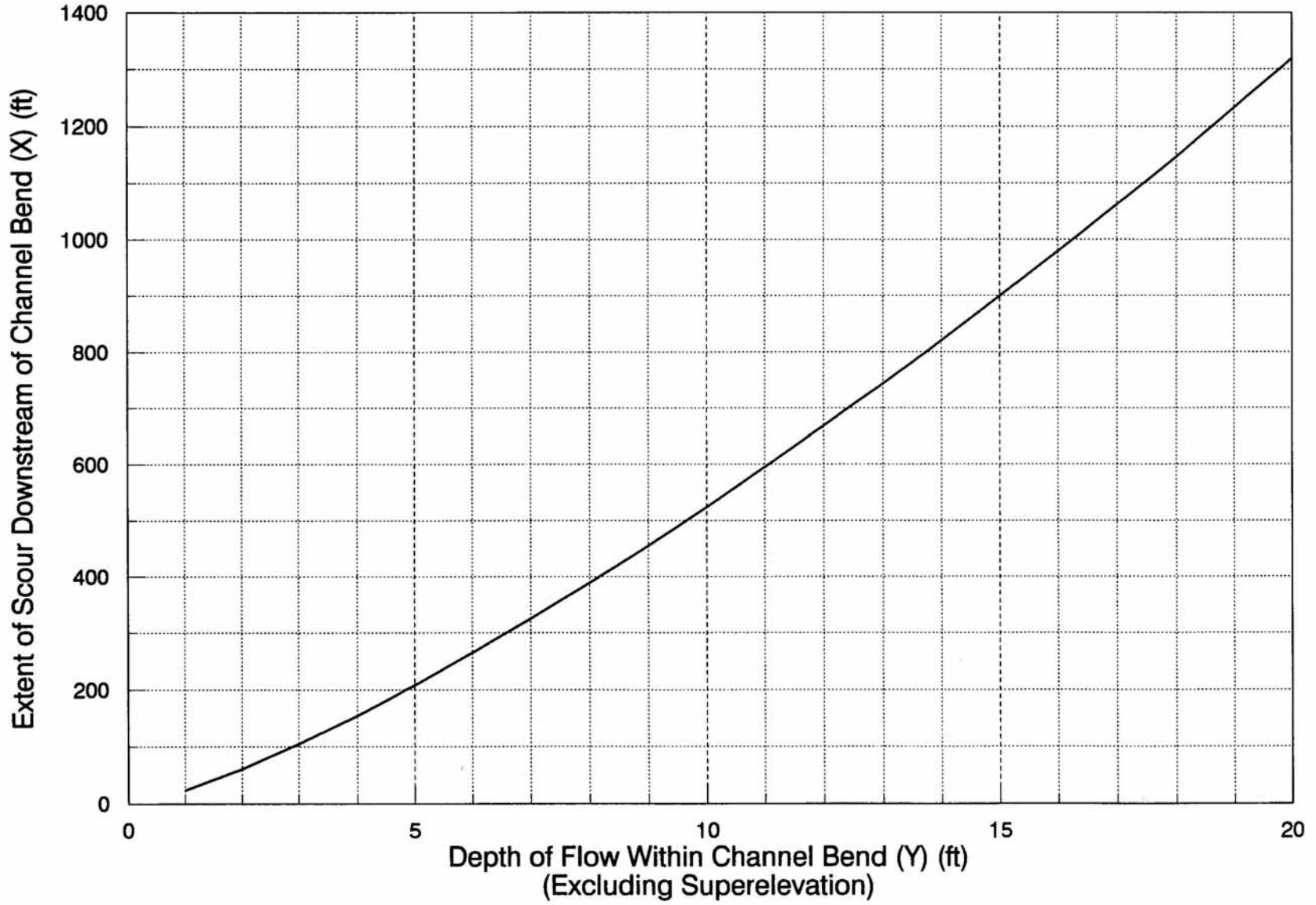


BEND SCOUR

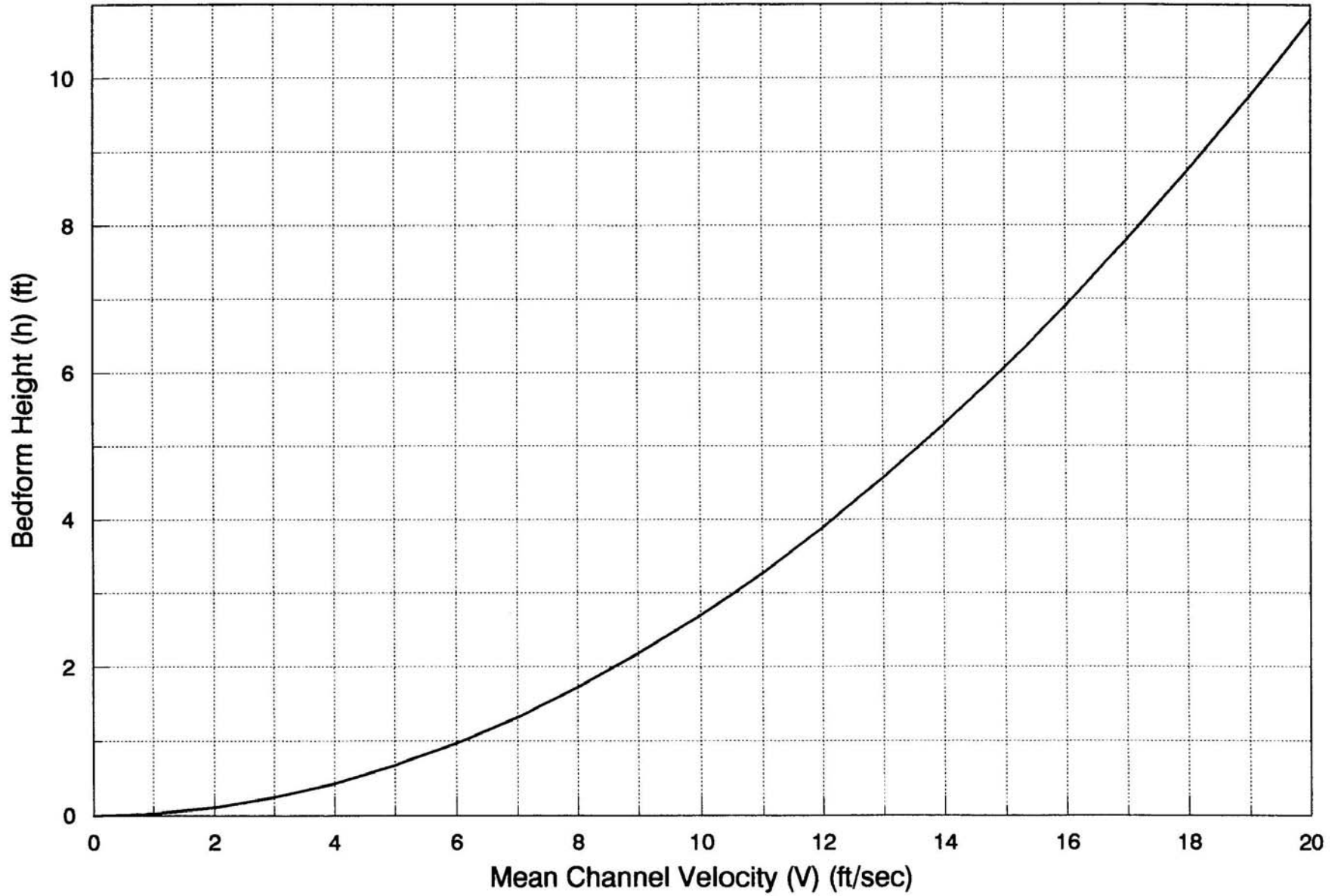
Energy Slope (S_e) = 0.001



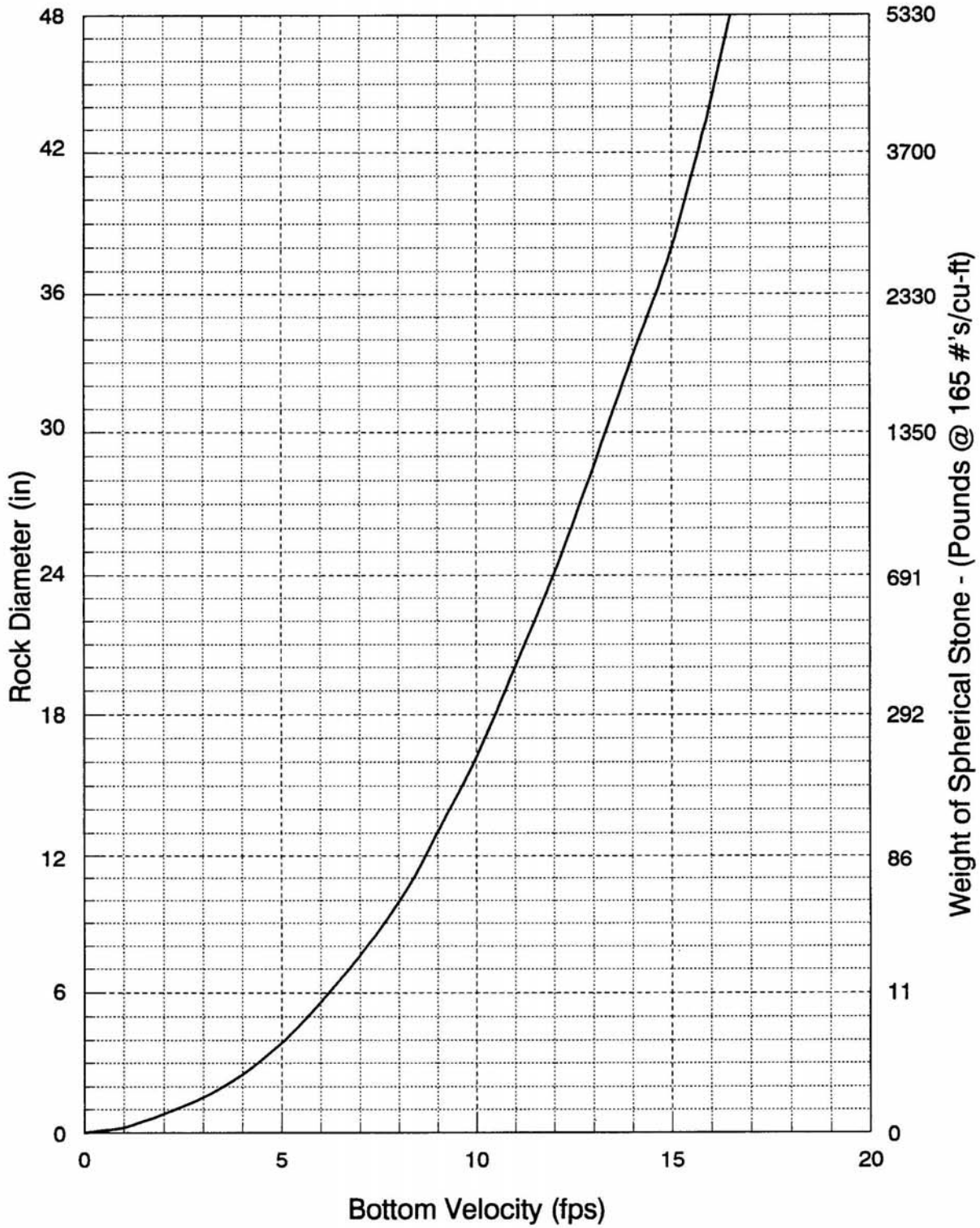
LONGITUDINAL EXTENT OF BEND SCOUR



BEDFORM HEIGHT



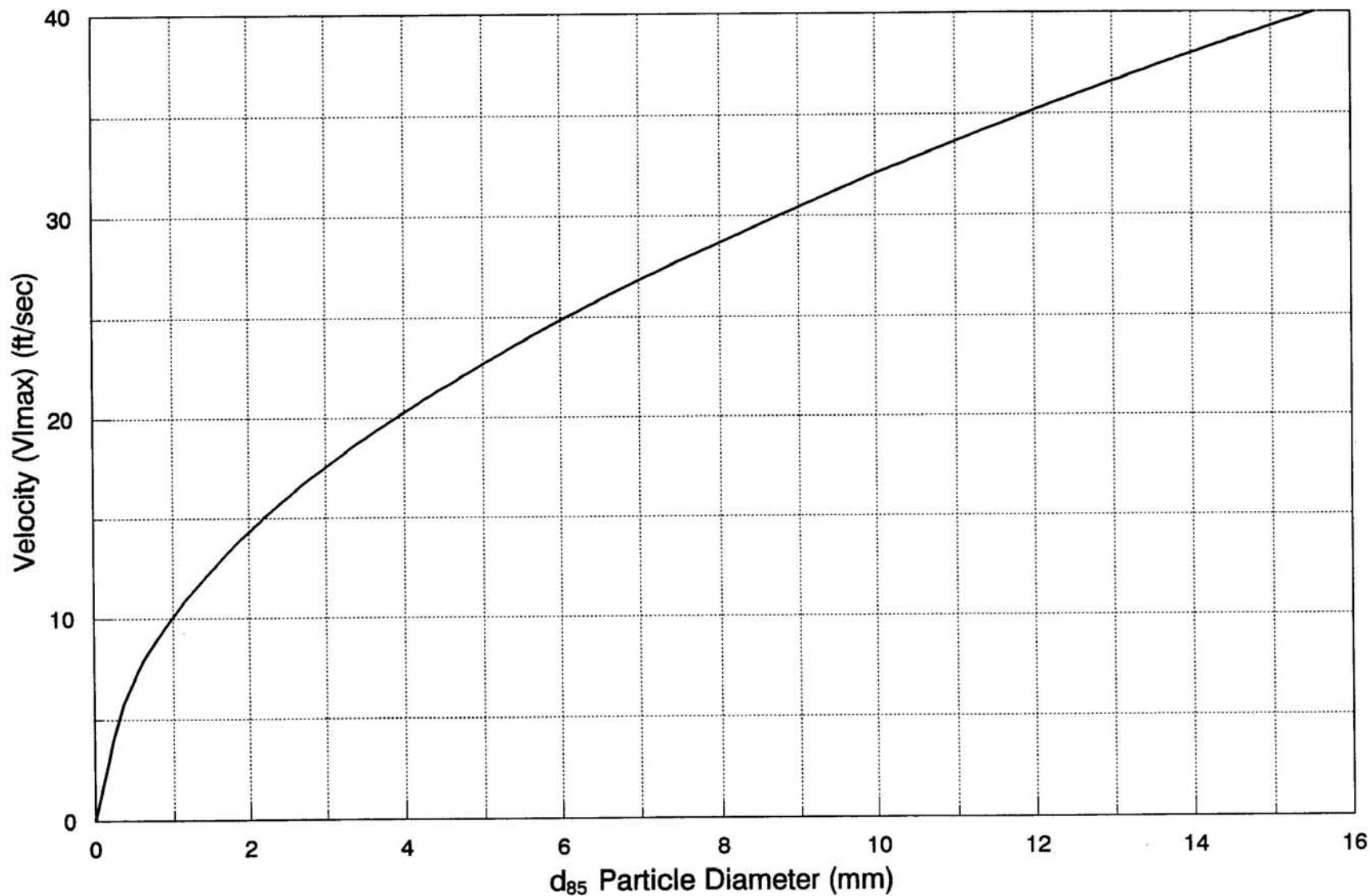
RIPRAP ROCK SIZE



For rock with specific gravity = 2.65

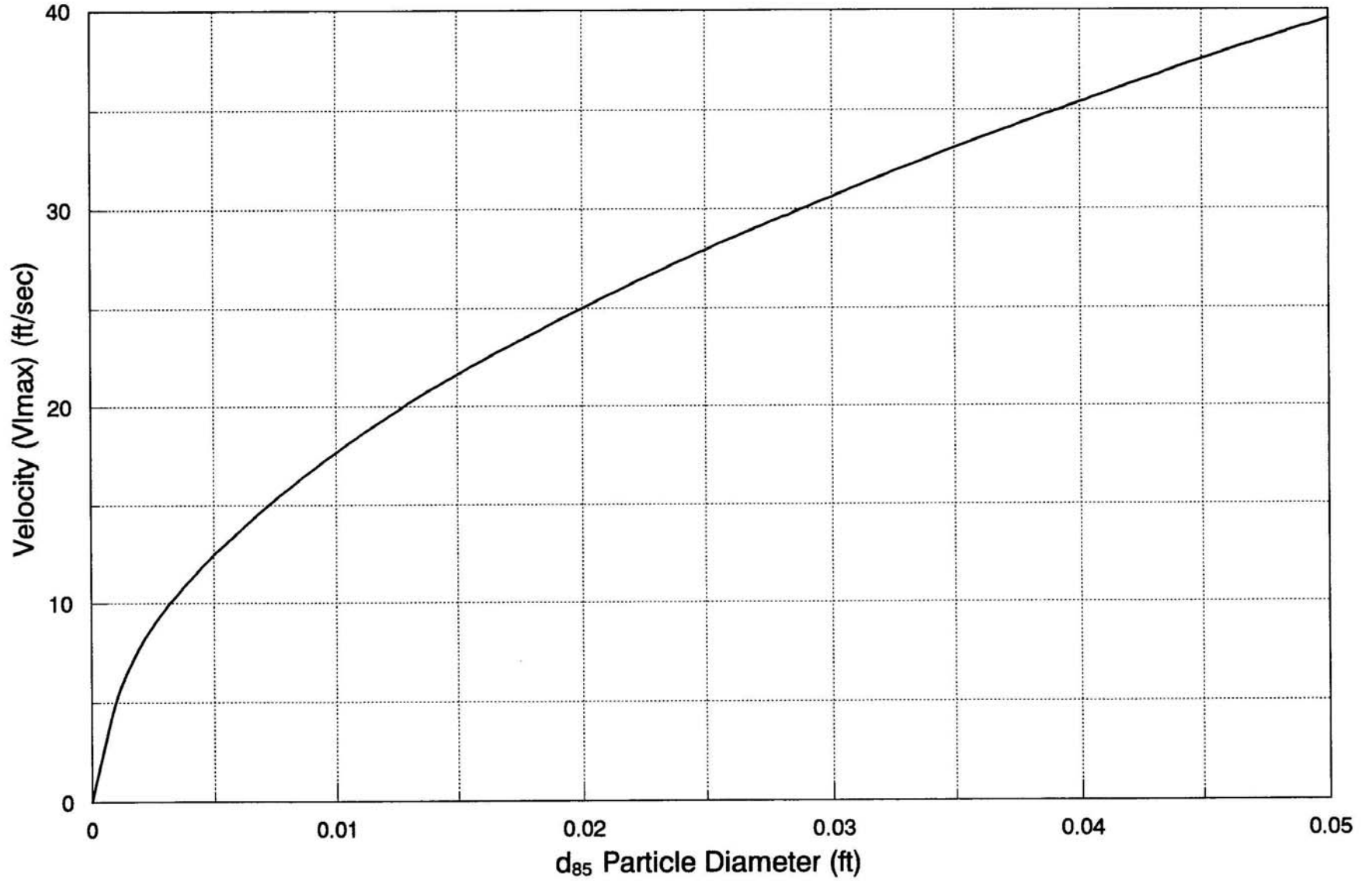
Source: Bureau of Reclamation

LIMITING DEPOSIT VELOCITY



d_{85} = The size of sediment, in mm, for which 85% of the sediment is finer

LIMITING DEPOSIT VELOCITY



d₈₅ = The size of sediment, in ft, for which 85% of the sediment is finer

SEDIMENTATION DESIGN CURVES SOURCE EQUATIONS

Pier Local Scour (Appendix page Q-3):

$$Z_{ls} = (1.04) b^{0.65} V^{0.43} Y^{0.135} \quad (\text{Neill, 1964})$$

where: Z_{ls} = Local scour depth at bridge piers, in feet
 b = Pier width normal to the direction of flow, in feet
 V = Upstream flow velocity, in ft/sec
 Y = Upstream flow depth, in feet.

Abutment Local Scour (Appendix page Q-5):

$$Z_{lsa} = (0.62) a^{0.40} V^{0.33} Y^{0.44} \quad (\text{Lin, 1961})$$

where: Z_{lsa} = Depth of local scour due to an abutment, in feet
 a = Length which abutment protrudes into the flow path, in feet
 V = Upstream flow velocity, in ft/sec
 Y = Upstream flow depth, in feet.

Bend Scour (Appendix page Q- 6A-C):

$$Z_{bs} = \frac{0.0685 Y V^{0.8}}{Y_h^{0.4} S_e^{0.3}} \left[1.59 \left(\frac{w}{r} \right)^{0.2} - 1 \right] \quad (\text{Zeller, 1981})$$

where: Z_{bs} = Bend scour component of total scour depth, in feet
 V = Mean velocity of upstream flow, in ft/sec
 Y = Maximum depth of upstream flow, in feet
 Y_h = Hydraulic depth of upstream flow, in feet
 S_e = Upstream energy slope
 w = Channel top width, in feet
 r = Radius of curvature to centerline of channel, in feet.

Longitudinal Extent of Bend Scour (Appendix page Q-7):

$$X = (0.41) C Y \quad (\text{Rozovski, 1961})$$

- where: X = Distance from the end of channel curvature to the downstream point at which secondary currents have dissipated, in feet
 C = Chezy coefficient (see equation below)
 Y = Depth of flow within the channel bend (not including scour), in feet.

The Chezy coefficient may be determined from the expression:

$$C = \frac{1.486}{n} R^{1/6}$$

- where: n = Manning roughness coefficient for the channel
 R = Hydraulic radius.

Bed form Height (Appendix page Q-8):

$$h = 0.14 \frac{2 \pi V^2}{g} = 0.027 V^2 \quad (\text{Kennedy, 1963})$$

- where: h = Bed form height
 V = Mean channel velocity
 g = Acceleration of gravity.

Limiting Deposit Velocity - d_{85} (Appendix pages Q- 10A-B):

In mm:

$$V_{lmax}^2 = 103 d_{85} \quad (\text{Sinclair, 1962})$$

where: V_{lmax} = Maximum limiting deposit velocity, in ft/sec
 d_{85} = The size of sediment for which 85% of the transported sediment is finer, in **mm**.

In feet:

$$V_{lmax}^2 = 31,300 d_{85} \quad (\text{Sinclair, 1962})$$

where: V_{lmax} = Maximum limiting deposit velocity, in ft/sec
 d_{85} = The size of sediment for which 85% of the transported sediment is finer, in **feet**.